

NorthWoods Software

Program Name: Weld-Group_HSSr

Project Number: -

Project Description: -

Project Designer: Dik

Last Revised (yy-mm-dd): 22.02.02

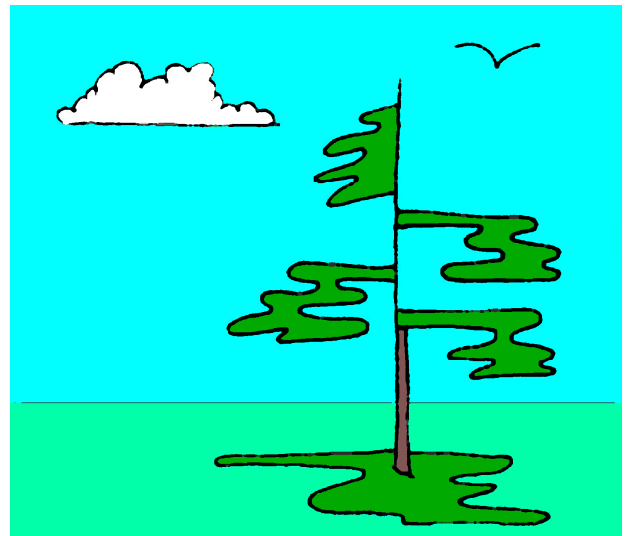
Reference: NBCC, CSA S16

Disclaimer:

Created using SMath Studio, a MathCAD workalike from <https://en.smath.info/view/SMathStudio>. The User is responsible to verify data and calculations using an alternative method

Menu:

..... Input Data	 Important Output	 Logical Constructs	Blue Units
..... Sum / For	Red Important Note	 Temporary Variables	



Input Data

Material Property Factors:

$$\phi_s := 0.90$$

$$\phi_w := 0.67$$

Load Factors:

$$\alpha_L := 1.50$$

$$\alpha_D := 1.25$$

$$\text{Check}(\alpha_D \geq 1.25) = "...OK"$$

$$\text{Check}(\alpha_L \geq 1.5) = "...OK"$$

Weld interface is taken at the face of the attached member rather than the centreline of the weld.

HSSr Shape Input Data:

$$d := 6 \text{ in}$$

$$b := 6 \text{ in}$$

$$c := .5 \text{ in}$$

$$d' := d - 2 \cdot c$$

$$b' := b - 2 \cdot c$$

Applied Factored Loads:

$$P_{fz} := 0 \text{ K}$$

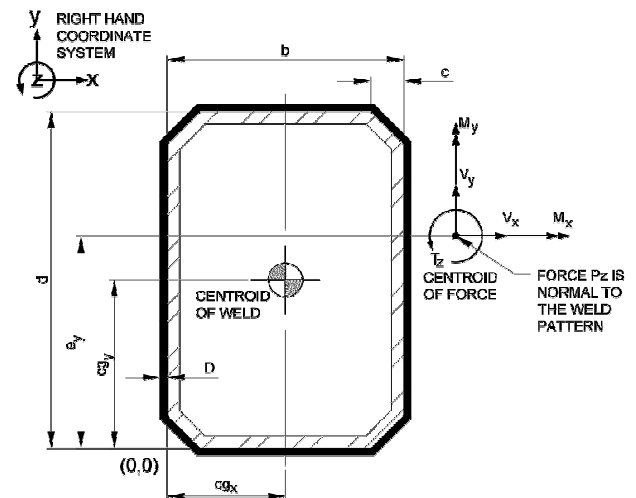
$$V_{fx} := 0 \text{ K}$$

$$V_{fy} := 25 \text{ K}$$

$$M_{fx} := 30 \text{ K_ft}$$

$$M_{fy} := 0 \text{ K_ft}$$

$$T_{fz} := 0 \text{ K_ft}$$



$$d = 152.4 \text{ mm}$$

Depth

$$b = 152.4 \text{ mm}$$

Width

$$c = 12.70 \text{ mm}$$

Chamfer Corner

$$P_{fz} = 0.00 \text{ kN}$$

Axial Load

$$V_{fx} = 0.00 \text{ kN}$$

Shear (X-Axis)

$$V_{fy} = 111.21 \text{ kN}$$

Shear (Y-Axis)

$$M_{fx} = 40.67 \text{ kN_m}$$

Moment (X-Axis)

$$M_{fy} = 0.00 \text{ kN_m}$$

Moment (Y-Axis)

$$T_{fz} = 0.00 \text{ kN_m}$$

Torsion (Z-Axis)

Load Eccentricity:

$$e_x := 3 \text{ in}$$

$$e_y := 3 \text{ in}$$

$$e_z := 0 \text{ in}$$

c**b**

$$e_x = 76.2 \text{ mm}$$

$$e_y = 76.2 \text{ mm}$$

$$e_z = 0.0 \text{ mm}$$

a

X-Axis Direction

Y-Axis Direction

Z-Axis Direction

Steel Properties:**Part:** $st_{NDX} := 1$

	NDX	des	fy	Fu	$desM_1 := st_{NDX} 2$	$f_{y1} := st_{NDX} 3$	$F_{u1} := st_{NDX} 4$
$st :=$	1	"G40.21-350W"	50 Ksi	65 Ksi	$G_s := \frac{E_s}{2 \cdot (1 + \nu)}$	$\gamma_s := 489 \text{ pcf}$	$\nu := 0.3$
	2	"G40.21-300W"	44 Ksi	65 Ksi			
	3	"A36"	36 Ksi	58 Ksi			

$E_s := 29000 \text{ Ksi}$

Base: $st_{NDX} := 2$

$$desM_2 := st_{NDX} 2$$

$$f_{y2} := st_{NDX} 3$$

$$F_{u2} := st_{NDX} 4$$

$$pts := \begin{bmatrix} s & x_i & y_i & x_j & y_j \\ 1 & c & 0 \text{ in} & 0 \text{ in} & c \\ 2 & 0 \text{ in} & c & 0 \text{ in} & c + d' \\ 3 & 0 \text{ in} & c + d' & c & d \\ 4 & c & d & c + b' & d \\ 5 & c + b' & d & b & c + d' \\ 6 & b & c + d' & b & c \\ 7 & b & c & c + b' & 0 \text{ in} \\ 8 & c + b' & 0 \text{ in} & c & 0 \text{ in} \end{bmatrix}$$

$$pts' := \text{submatrix}(pts, 1, \text{rows}(pts), 2, 5)$$

```
for c ∈ [1..rows(pts')]
  ptsn_c1 := pts' c 1 . 1/in
  ptsn_c2 := pts' c 2 . 1/in
  ptsn_c3 := pts' c 3 . 1/in
  ptsn_c4 := pts' c 4 . 1/in
```

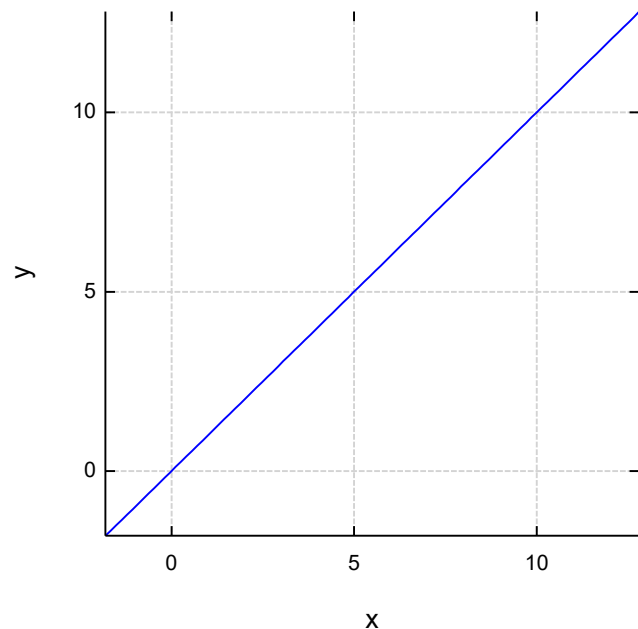
$$c := 0$$

```
for c ∈ [1..rows(ptsn)]
  weld_c1 := ptsn c 1
  weld_c2 := ptsn c 2
```

$$c := c + 1$$

$$weld_{c1} := ptsn_{11}$$

$$weld_{c2} := ptsn_{12}$$

Weld Pattern

weld

Weld Design:**Weld Electrodes:**

$$we_{NDX} := 2$$

	NDX	Desl	DesM	UTS	
$we :=$	1	"E60xx"	"E43xx"	60 ksi	$desI_w := we_{we_{NDX} 2}$
	2	"E70xx"	"E49xx"	70 ksi	$desM_w := we_{we_{NDX} 3}$
	3	"E80xx"	"E55xx-x"	80 ksi	$X_u := we_{we_{NDX} 4}$
	4	"E90xx"	"E62xx-x"	90 ksi	

Weld Sizes:

$$ws_{NDX} := 13$$

	NDX	desl	desM	D	
$ws :=$	1	" "	"3mm"	0.11811 in	$desI_D := ws_{ws_{NDX} 2}$
	2	"1/8"	" "	0.125 in	$desM_D := ws_{ws_{NDX} 3}$
	3	" "	"4mm"	0.15748 in	$D := ws_{ws_{NDX} 4}$
	4	"3/16"	" "	0.1875 in	
	5	" "	"5mm"	0.19685 in	
	6	" "	"6mm"	0.23622 in	
	7	"1/4"	" "	0.25 in	
	8	" "	"7mm"	0.275591 in	
	9	"5/16"	" "	0.3125 in	
	10	" "	"8mm"	0.314961 in	
	11	" "	"9mm"	0.354331 in	
	12	"3/8"	" "	0.375 in	
	13	" "	"10mm"	0.3937 in	
	14	" "	"11mm"	0.4331 in	
	15	"7/16"	" "	0.4375 in	
	16	" "	"12mm"	0.4724 in	
	17	"1/2"	" "	0.50 in	

Weld Material Strength

$$v_{rw} := \phi_w \cdot 0.67 \cdot X_u \cdot \frac{1}{\sqrt{2}} \quad v_{rw} = 22.2 \text{ ksi} \quad v_{rw} = 153.2 \text{ MPa} \quad \text{Weld Strength}$$

HSSr Metal Strength

$$v_{rb1} := \phi_w \cdot 0.67 \cdot F_{u1} \quad v_{rb1} = 29.2 \text{ Ksi} \quad v_{rb1} = 201.2 \text{ MPa} \quad \text{HSSr Strength}$$

Base Metal Strength

$$v_{rb2} := \phi_w \cdot 0.67 \cdot F_{u1} \quad v_{rb2} = 29.2 \text{ Ksi} \quad v_{rb2} = 201.2 \text{ MPa} \quad \text{Base Metal Strength}$$

Minimum Weld Capacity

$$V_{rw} := \min \left(\left[v_{rw} \quad v_{rb1} \quad v_{rb2} \right] \right) \quad V_{rw} = 22.2 \text{ Ksi} \quad V_{rw} = 153.2 \text{ MPa} \quad \text{Weld Resistance}$$

Weld Properties:

```

for r ∈ [1..rows(pts)]
  pts_r6 := (pts_r2 + pts_r4) / 2
  pts_r7 := (pts_r3 + pts_r5) / 2
  pts_r8 := pts_r4 - pts_r2
  pts_r9 := pts_r5 - pts_r3
  pts_r10 := sqrt((pts_r8)^2 + (pts_r9)^2)
  pts_r11 := (pts_r10 * (pts_r9)^2) / 12
  pts_r12 := (pts_r10 * (pts_r8)^2) / 12
  pts_r13 := pts_r10 * pts_r6
  pts_r14 := pts_r10 * pts_r7

```

c_x
 c_y
 m
 n
 L
 I_{xx}
 I_{yy}
 L_{cx}
 L_{cy}

```

for r ∈ [1..rows(pts)]
  pts_r15 := pts_r6 - cg_x
  pts_r16 := pts_r7 - cg_y
  pts_r17 := pts_r11 + pts_r10 * pts_r16^2
  pts_r18 := pts_r12 + pts_r10 * (pts_r15)^2
  pts_r19 := pts_r10 * pts_r15 * pts_r16
  pts_r20 := pts_r2 - cg_x
  pts_r21 := pts_r3 - cg_y
  pts_r22 := pts_r4 - cg_x
  pts_r23 := pts_r5 - cg_y

```

d_x
 d_y
 I'_{xx}
 I'_{yy}
 I'_{xy}
 d_{xi}
 d_{yi}
 d_{xj}
 d_{yj}

$A := \sum \text{col}(pts, 10)$	$A = 22.83 \text{ in}$	$A = 579.8 \text{ mm}$	Total Length of Weld
$cg_x := \sum \left(\frac{\text{col}(pts, 13)}{A} \right)$	$cg_x = 3.00 \text{ in}$	$cg_x = 76.2 \text{ mm}$	Centroid of Weld X-Axis
$cg_y := \sum \left(\frac{\text{col}(pts, 14)}{A} \right)$	$cg_y = 3.00 \text{ in}$	$cg_y = 76.2 \text{ mm}$	Centroid of Weld Y-Axis
$I'_{xx} := \sum \text{col}(pts, 17)$	$I'_{xx} = 132.3 \text{ in}^3$	$I'_{xx} = 2.17 \cdot 10^6 \text{ mm}^3$	Moment of Inertia (X-Axis)
$I'_{yy} := \sum \text{col}(pts, 18)$	$I'_{yy} = 132.3 \text{ in}^3$	$I'_{yy} = 2.17 \cdot 10^6 \text{ mm}^3$	Moment of Inertia (Y-Axis)
$I'_{xy} := \sum \text{col}(pts, 19)$	$I'_{xy} = 3.9 \cdot 10^{-14} \text{ in}^3$	$I'_{xy} = 6.43 \cdot 10^{-10} \text{ mm}^3$	Product of Inertia
$I'_p := I'_{xx} + I'_{yy}$	$I'_p = 264.6 \text{ in}^3$	$I'_p = 4.34 \cdot 10^6 \text{ mm}^3$	Polar Moment of Inertia
$I'_{prod} := I'_{xx} \cdot I'_{yy} - I'_{xy}^2$	$I'_{prod} = 17498.6 \text{ in}^6$	$I'_{prod} = 4.70 \cdot 10^{12} \text{ mm}^6$	Product of Inertia

Load Distance from Centroid:

$$e'_x := e_x - cg_x$$

$$e'_y := e_y - cg_y$$

$$e'_z := e_z$$

$$e'_x = -1.43 \cdot 10^{-14} \text{ in} \quad e'_x = -3.6276 \cdot 10^{-13} \text{ mm} \quad \text{X-Axis Direction}$$

$$e'_y = -1.06 \cdot 10^{-14} \text{ in} \quad e'_y = -2.6923 \cdot 10^{-13} \text{ mm} \quad \text{Y-Axis Direction}$$

$$e'_z = 0.00 \text{ in} \quad e'_z = 0 \text{ mm} \quad \text{Z-Axis Direction}$$

Stress from Applied Factored Loads:

$$\sigma_{fx} := \frac{V_{fx}}{A}$$

$$\sigma_{fx} = 0.00 \text{ Kpi}$$

$$\sigma_{fx} = 0 \text{ kNpmm}$$

X-Axis Direction

$$\sigma_{fy} := \frac{V_{fy}}{A}$$

$$\sigma_{fy} = 1.0951 \text{ Kpi}$$

$$\sigma_{fy} = 0.1918 \text{ kNpmm}$$

Y-Axis Direction

$$\sigma_{fz} := \frac{P_{fz}}{A}$$

$$\sigma_{fz} = 0 \text{ Kpi}$$

$$\sigma_{fz} = 0 \text{ kNpmm}$$

Z-Axis Direction

Load at Weld Group Centroid:

$$d_x := e_x - cg_x$$

$$d_x = -1.43 \cdot 10^{-14} \text{ in} \quad d_x = -3.63 \cdot 10^{-13} \text{ mm}$$

$$d_y := e_y - cg_y$$

$$d_y = -1.06 \cdot 10^{-14} \text{ in} \quad d_y = -2.69 \cdot 10^{-13} \text{ mm}$$

$$d_z := e_z$$

$$d_z = 0 \text{ in} \quad d_z = 0 \text{ mm}$$

$$M'_{fx} := M_{fx} - V_{fy} \cdot d_z + P_{fz} \cdot d_y$$

$$M'_{fx} = 30.00 \text{ K_ft}$$

$$M'_{fx} = 40.67 \text{ kN_m}$$

$$M'_{fy} := M_{fy} + V_{fx} \cdot d_z - P_{fz} \cdot d_x$$

$$M'_{fy} = 0 \text{ K_ft}$$

$$M'_{fy} = 0 \text{ kN_m}$$

$$T'_{fz} := T_{fz} - V_{fx} \cdot d_y + V_{fy} \cdot d_x$$

$$T'_{fz} = -2.98 \cdot 10^{-14} \text{ K_ft} \quad T'_{fz} = -4.03 \cdot 10^{-14} \text{ kN_m}$$

```
for r ∈ [1..rows(pts)]
```

```
  if ptsr 10 = 0 in
```

```
    ptsr 24 := 0 pli
```

```
    ptsr 25 := 0 pli
```

```
    ptsr 26 := 0 pli
```

```
    ptsr 27 := 0 pli
```

```
    ptsr 28 := ptsr 24 + ptsr 25 + ptsr 26 + ptsr 27
```

```
    ptsr 29 := ptsr 28 +  $\sigma_{fz}$ 
```

```
    ptsr 30 := 0 pli
```

```
    ptsr 31 := ptsr 30 +  $\sigma_{fx}$ 
```

```
    ptsr 32 := 0 pli
```

```
    ptsr 33 := ptsr 32 +  $\sigma_{fy}$ 
```

```
    ptsr 34 :=  $\sqrt{pts_{r 29}^2 + pts_{r 31}^2 + pts_{r 33}^2}$ 
```

```
    ptsr 35 := 0 pli
```

```
  else
```

```
    ptsr 24 :=  $\frac{pts_{r 21} \cdot M'_{fx} \cdot I'_{yy}}{I'_{prod}}$ 
```

```
    ptsr 25 :=  $\frac{pts_{r 21} \cdot M'_{fy} \cdot I'_{xy}}{I'_{prod}}$ 
```

```
    ptsr 26 :=  $\frac{(-1 \cdot pts_{r 20}) \cdot M'_{fx} \cdot I'_{xy}}{I'_{prod}}$ 
```

```
    ptsr 27 :=  $\frac{(-1 \cdot pts_{r 20}) \cdot M'_{fy} \cdot I'_{xx}}{I'_{prod}}$ 
```

```
    ptsr 28 := ptsr 24 + ptsr 25 + ptsr 26 + ptsr 27
```

```
    ptsr 29 := ptsr 28 +  $\sigma_{fz}$ 
```

```
    ptsr 30 :=  $\frac{(-1 \cdot pts_{r 21}) \cdot T'_{fz}}{I'_p}$ 
```

```
    ptsr 31 := ptsr 30 +  $\sigma_{fx}$ 
```

```
    ptsr 32 :=  $\frac{pts_{r 20} \cdot T'_{fz}}{I'_p}$ 
```

```
    ptsr 33 := ptsr 32 +  $\sigma_{fy}$ 
```

```
    ptsr 34 :=  $\sqrt{(pts_{r 29})^2 + (pts_{r 31})^2 + (pts_{r 33})^2}$ 
```

```
    ptsr 35 :=  $\frac{pts_{r 23} \cdot M'_{fx} \cdot I'_{yy}}{I'_{prod}}$ 
```

σ_z y from Mx

Stress at Start of Segment

σ_z y from My

σ_z x from Mx

σ_z x from My

$\Sigma \sigma_z$ from M

$\Sigma \sigma_z$

σ_x from T

$\Sigma \sigma_x$

σ_y from T

$\Sigma \sigma_y$

σ_y combined

σ_z y from Mx

σ_z y from My

σ_z x from Mx

σ_z x from My

$\Sigma \sigma_z$ from M

$\Sigma \sigma_z$

σ_x from T

$\Sigma \sigma_x$

σ_y from T

$\Sigma \sigma_y$

σ_y combined

$\Sigma \sigma_y$

Stress at Start of Segment

σ_z y from Mx

Stress at End of Segment

```
for r ∈ [1..rows(pts)]
```

```
  if ptsr 10 = 0 in
```

```
    ptsr 36 := 0 pli
```

```
    ptsr 37 := 0 pli
```

```
    ptsr 38 := 0 pli
```

```
    ptsr 39 := ptsr 35 + ptsr 36 + ptsr 37 + ptsr 38
```

```
    ptsr 40 := (ptsr 39 + σfz)
```

```
    ptsr 41 := 0 pli
```

```
    ptsr 42 := ptsr 41 + σfx
```

```
    ptsr 43 := 0 pli
```

```
    ptsr 44 := ptsr 43 + σfy
```

```
    ptsr 45 := √((ptsr 40)2 + (ptsr 42)2 + (ptsr 44)2)
```

```
    ptsr 46 := 0 pli
```

```
    ptsr 47 := 0 pli
```

```
    ptsr 48 := 0 pli
```

```
  else
```

```
    ptsr 36 :=  $\frac{pts_{r 23} \cdot M'_{fy} \cdot I'_{xy}}{I'_{prod}}$ 
```

```
    ptsr 37 :=  $\frac{(-1 \cdot pts_{r 22}) \cdot M'_{fx} \cdot I'_{xy}}{I'_{prod}}$ 
```

```
    ptsr 38 :=  $\frac{(-1 \cdot pts_{r 22}) \cdot M'_{fy} \cdot I'_{xx}}{I'_{prod}}$ 
```

```
    ptsr 39 := ptsr 35 + ptsr 36 + ptsr 37 + ptsr 38
```

```
    ptsr 40 := (ptsr 39 + σfz)
```

```
    ptsr 41 :=  $\left( - \frac{1 \cdot pts_{r 23} \cdot T'_{fz}}{I'_p} \right)$ 
```

```
    ptsr 42 := ptsr 41 + σfx
```

```
    ptsr 43 :=  $\frac{pts_{r 22} \cdot T'_{fz}}{I'_p}$ 
```

```
    ptsr 44 := ptsr 43 + σfy
```

```
    ptsr 45 := √((ptsr 40)2 + (ptsr 42)2 + (ptsr 44)2)
```

```
    ptsr 46 :=  $\frac{pts_{r 16} \cdot M'_{fx} \cdot I'_{yy}}{I'_{prod}}$ 
```

```
    ptsr 47 :=  $\frac{pts_{r 16} \cdot M'_{fy} \cdot I'_{xy}}{I'_{prod}}$ 
```

```
    ptsr 48 :=  $\frac{(-1 \cdot pts_{r 15} \cdot M'_{fx} \cdot I'_{xy})}{I'_{prod}}$ 
```

σz y from My

σz x from Mx

σz x from My

Σσz from M

Σσz

σx from T

Σσx

σy from T

Σσy

σy combined

σz y from Mx

σz y from My

σz x from Mx

σz y from My

σz x from Mx

σz x from My

Σσz from M

Σσz

σx from T

Σσx

σy from T

Σσy

σy combined

Stress at End of Segment

σz y from Mx

Stress at Midpoint of Segment

σz y from My

σz x from Mx

<pre> for r ∈ [1..rows(pts)] if pts_r 10 = 0 in pts_r 49 := 0 pli pts_r 50 := pts_r 46 + pts_r 47 + pts_r 48 + pts_r 49 pts_r 51 := pts_r 50 + σ_{fz} pts_r 52 := 0 pli pts_r 53 := pts_r 52 · σ_{fx} pts_r 54 := 0 pli pts_r 55 := pts_r 54 + σ_{fy} pts_r 56 := √((pts_r 51)² + (pts_r 53)² + (pts_r 55)²) else pts_r 49 := $\frac{(-1 \cdot pts_r 15) \cdot M'_{fy} \cdot I'_{xx}}{I'_{prod}}$ pts_r 50 := pts_r 46 + pts_r 47 + pts_r 48 + pts_r 49 pts_r 51 := pts_r 50 + σ_{fz} pts_r 52 := $\frac{(-1 \cdot pts_r 16) \cdot T'_{fz}}{I'_p}$ pts_r 53 := (pts_r 52 + σ_{fx}) pts_r 54 := $\frac{pts_r 15 \cdot T'_{fz}}{I'_p}$ pts_r 55 := pts_r 54 + σ_{fy} pts_r 56 := √((pts_r 51)² + (pts_r 53)² + (pts_r 55)²) </pre>			σ _z x from My	
			σ _z frin M	
			σ _z Total	
			σ _x from T	
			σ _x Combined	
			σ _y from T	
			σ _y Combined	
			Total σ	
			σ _z x from My	
			σ _z frin M	
			σ _z Total	
			σ _x from T	
			σ _x Combined	
			σ _y from T	
			σ _y Combined	
			Total σ	Stress at Midpoint of Segment

Max and Min Stress in Segment:

$\sigma'_z := \max(\text{col}(\text{pts}, 34))$	$\sigma'_z = 8.24 \text{ Kpi}$	$\sigma'_z = 1.44 \text{ kNpmm}$	Max Stress at Start Segment
$op'_1 := \min(\text{col}(\text{pts}, 34))$	$op'_1 = 6.89 \text{ Kpi}$	$op'_1 = 1.21 \text{ kNpmm}$	Min Stress at Start Segment
$\sigma'_x := \max(\text{col}(\text{pts}, 45))$	$\sigma'_x = 8.24 \text{ Kpi}$	$\sigma'_x = 1.44 \text{ kNpmm}$	Max Stress at End Segment
$op'_2 := \min(\text{col}(\text{pts}, 45))$	$op'_2 = 6.89 \text{ Kpi}$	$op'_2 = 1.21 \text{ kNpmm}$	Min Stress at End Segment
$\sigma'_y := \max(\text{col}(\text{pts}, 56))$	$\sigma'_y = 8.24 \text{ Kpi}$	$\sigma'_y = 1.44 \text{ kNpmm}$	Max Stress at Mid Segment
$op'_3 := \min(\text{col}(\text{pts}, 56))$	$op'_3 = 1.1 \text{ Kpi}$	$op'_3 = 0.19 \text{ kNpmm}$	Min Stress at Mid Segment

Max and Min Stress in Welds:

$op_{max} := \max\left(\left[\begin{matrix} \sigma'_x & \sigma'_y & \sigma'_z \end{matrix}\right]\right)$	$op_{max} = 8.24 \text{ Kpi}$	$op_{max} = 1.44 \text{ kNpmm}$	Max Stress in Segment
$op'_{min} := \min\left(\left[\begin{matrix} op'_1 & op'_2 & op'_3 \end{matrix}\right]\right)$	$op'_{min} = 1.10 \text{ Kpi}$	$op'_{min} = 0.19 \text{ kNpmm}$	Min Stress in Segment
$v_r := v_{rw} \cdot D$	$v_r = 8.75 \text{ Kpi}$	$v_r = 1.532 \text{ kNpmm}$	Weld Resistance per Unit Length

Summary:**Material Property Factors**

$$\text{Rolled Sections} \quad \phi_s = 0.9$$

$$\text{Weld} \quad \phi_w = 0.67$$

$$\text{Dead Load Factor} \quad \alpha_D = 1.25$$

$$\text{Live Load Factor} \quad \alpha_L = 1.5$$

$$\text{Check } (\alpha_D \geq 1.25) = "...OK"$$

$$\text{Check } (\alpha_L \geq 1.5) = "...OK"$$

Applied Factored Loads:

Axial Load

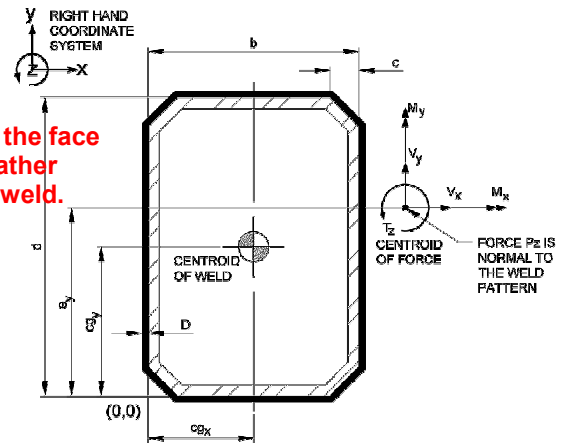
Shear (X-Axis)

Shear (Y-Axis)

Moment (X-Axis)

Moment (Y-Axis)

Torsion (Z-Axis)



Weld interface is taken at the face of the attached member rather than the centreline of the weld.

$$P_{fz} = 0 \text{ K}$$

$$V_{fx} = 0 \text{ K}$$

$$V_{fy} = 25 \text{ K}$$

$$M_{fx} = 30 \text{ K_ft}$$

$$M_{fy} = 0 \text{ K_ft}$$

$$T_{fz} = 0 \text{ K_ft}$$

$$P_{fz} = 0 \text{ kN}$$

$$V_{fx} = 0 \text{ kN}$$

$$V_{fy} = 111.2055 \text{ kN}$$

$$M_{fx} = 40.6745 \text{ kN_m}$$

$$M_{fy} = 0 \text{ kN_m}$$

$$T_{fz} = 0 \text{ kN_m}$$

Load Eccentricity from Origin:

X-Axis Direction

Y-Axis Direction

Z-Axis Direction

$$e_x = 3 \text{ in}$$

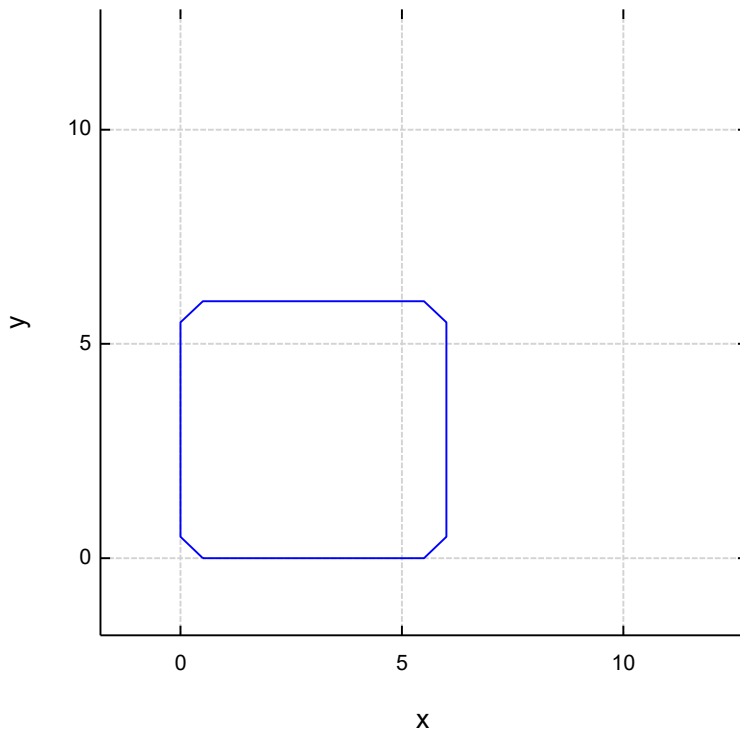
$$e_y = 3 \text{ in}$$

$$e_z = 0 \text{ in}$$

$$e_x = 76.2 \text{ mm}$$

$$e_y = 76.2 \text{ mm}$$

$$e_z = 0 \text{ mm}$$

Weld Pattern

weld

Segment Input Data:

Row Represents Segment
Number Columns represent
S(xi, yi, xj, yj) coordinates

$$pts' = \begin{bmatrix} 0.5 & 0.0 & 0.0 & 0.5 \\ 0.0 & 0.5 & 0.0 & 5.5 \\ 0.0 & 5.5 & 0.5 & 6.0 \\ 0.5 & 6.0 & 5.5 & 6.0 \\ 5.5 & 6.0 & 6.0 & 5.5 \\ 6.0 & 5.5 & 6.0 & 0.5 \\ 6.0 & 0.5 & 5.5 & 0.0 \\ 5.5 & 0.0 & 0.5 & 0.0 \end{bmatrix} \text{ in}$$

$$pts' = \begin{bmatrix} 12.7 & 0 & 0 & 12.7 \\ 0 & 12.7 & 0 & 139.7 \\ 0 & 139.7 & 12.7 & 152.4 \\ 12.7 & 152.4 & 139.7 & 152.4 \\ 139.7 & 152.4 & 152.4 & 139.7 \\ 152.4 & 139.7 & 152.4 & 12.7 \\ 152.4 & 12.7 & 139.7 & 0 \\ 139.7 & 0 & 12.7 & 0 \end{bmatrix} \text{ mm}$$

Steel Properties**Part**

Designation

$$desM_1 = \text{"G40.21-350W"}$$

Yield Strength

$$f_{y1} = 50 \text{ Ksi}$$

$$f_{y1} = 345 \text{ MPa}$$

Ultimate Strength

$$F_{u1} = 65 \text{ Ksi}$$

$$F_{u1} = 448 \text{ MPa}$$

Base

Designation

$$desM_2 = \text{"G40.21-300W"}$$

Yield Strength

$$f_{y2} = 44 \text{ Ksi}$$

$$f_{y2} = 303 \text{ MPa}$$

Ultimate Strength

$$F_{u2} = 65 \text{ Ksi}$$

$$F_{u2} = 448 \text{ MPa}$$

Young's Modulus

$$E_s = 29000 \text{ Ksi}$$

$$E_s = 1.9995 \cdot 10^5 \text{ MPa}$$

Shear Modulus

$$G_s = 11153.8462 \text{ Ksi}$$

$$G_s = 76903 \text{ MPa}$$

Density

$$\gamma_s = 489 \text{ pcf}$$

$$\gamma_s = 76.8158 \text{ kNpcm}$$

Welding Electrodes

Imperial Designation

$$desI_w = \text{"E70xx"}$$

Metric Designation

$$desM_w = \text{"E49xx"}$$

Ultimate Strength

$$X_u = 70 \text{ ksi}$$

$$X_u = 483 \text{ MPa}$$

Weld Size

Imperial Designation

$$desI_D = \text{" "}$$

Metric Designation

$$desM_D = \text{"10mm"}$$

Size

$$D = 0.3937 \text{ in}$$

$$D = 10 \text{ mm}$$

Weld Strength

$$v_{rw} = 22.2 \text{ ksi}$$

$$v_{rw} = 153.2 \text{ MPa}$$

Part to be Welded Strength

$$v_{rb1} = 29.2 \text{ Ksi}$$

$$v_{rb1} = 201.2 \text{ MPa}$$

Base Metal Strength

$$v_{rb2} = 29.2 \text{ Ksi}$$

$$v_{rb2} = 201.2 \text{ MPa}$$

Weld Resistance

$$V_{rw} = 22.2 \text{ Ksi}$$

$$V_{rw} = 153.2 \text{ MPa}$$

Total Length of Weld

$$A = 22.8284 \text{ in}$$

$$A = 579.8 \text{ mm}$$

Centroid of Weld X-Axis

$$cg_x = 3 \text{ in}$$

$$cg_x = 76.2 \text{ mm}$$

Centroid of Weld Y-Axis

$$cg_y = 3 \text{ in}$$

$$cg_y = 76.2 \text{ mm}$$

Moment of Inertia (X-Axis)

$$I'_{xx} = 132.3 \text{ in}^3$$

$$I'_{xx} = 2.17 \cdot 10^6 \text{ mm}^3$$

Moment of Inertia (Y-Axis)

$$I'_{yy} = 132.3 \text{ in}^3$$

$$I'_{yy} = 2.17 \cdot 10^6 \text{ mm}^3$$

Product of Inertia

$$I'_{xy} = 3.9 \cdot 10^{-14} \text{ in}^3$$

$$I'_{xy} = 6.43 \cdot 10^{-10} \text{ mm}^3$$

Polar Moment of Inertia

$$I'_p = 264.6 \text{ in}^3$$

$$I'_p = 4.34 \cdot 10^6 \text{ mm}^3$$

Prod of Moment of Inertia

$$I'_{prod} = 17498.6 \text{ in}^6$$

$$I'_{prod} = 4.70 \cdot 10^{12} \text{ mm}^6$$

Weld Force from Direct Loads

X-Axis Direction

$$\sigma_{fx} = 0.00 \text{ Kpi}$$

$$\sigma_{fx} = 0.00 \text{ kNpmm}$$

Y-Axis Direction

$$\sigma_{fy} = 1.10 \text{ Kpi}$$

$$\sigma_{fy} = 0.19 \text{ kNpmm}$$

Z-Axis Direction

$$\sigma_{fz} = 0.00 \text{ Kpi}$$

$$\sigma_{fz} = 0.00 \text{ kNpmm}$$

Load at Weld Group Centroid:

Moment (X-Axis)

$$M'_{fx} = 30 \text{ K_ft}$$

$$M'_{fx} = 40.6745 \text{ kN_m}$$

Moment (Y-Axis)

$$M'_{fy} = 0.00 \text{ K_ft}$$

$$M'_{fy} = 0.00 \text{ kN_m}$$

Torsion (Z-Axis)

$$T'_{fz} = -2.98 \cdot 10^{-14} \text{ K_ft}$$

$$T'_{fz} = -4.03 \cdot 10^{-14} \text{ kN_m}$$

Max Weld Force at Start of Segment in Pattern

$$\sigma'_x = 8.237 \text{ Kpi}$$

$$\sigma'_x = 1.44 \text{ kNpmm}$$

Min Weld Force at Start of Segment in Pattern

$$op'_1 = 6.891 \text{ Kpi}$$

$$op'_1 = 1 \text{ kNpmm}$$

Max Weld Force at End of Segment in Pattern

$$\sigma'_y = 8.237 \text{ Kpi}$$

$$\sigma'_y = 1.44 \text{ kNpmm}$$

Min Weld Force at End of Segment in Pattern

$$op'_2 = 6.891 \text{ Kpi}$$

$$op'_2 = 1.21 \text{ kNpmm}$$

Max Weld Force at Mid Point of Segment in Pattern

$$\sigma'_z = 8.237 \text{ Kpi}$$

$$\sigma'_z = 1.44 \text{ kNpmm}$$

Min Weld Force at Mid Point of Segment in Pattern

$$op'_3 = 1.095 \text{ Kpi}$$

$$op'_3 = 0.19 \text{ kNpmm}$$

Maximum Weld Force in Pattern

$$op_{max} = 8.237 \text{ Kpi}$$

$$op_{max} = 1.44 \text{ kNpmm}$$

Minimum Weld Force in Pattern

$$op'_{min} = 1.095 \text{ Kpi}$$

$$op'_{min} = 0.19 \text{ kNpmm}$$

Weld Resistance:

$$v_r = 8.748 \text{ Kpi}$$

$$v_r = 1.53 \text{ kNpmm}$$

$$Check \left(v_r \geq 0.95 \cdot op_{max} \right) = "...OK"$$

$$Check \left(v_r \geq op_{max} \right) = "...OK"$$

$$Check \left(v_r \geq |op'_{min}| \right) = "...OK"$$