

NorthWoods Software

Program Name: Comp-W_C_1W

Project Number: -

Project Description: -

Project Designer: Dik

Last Revised (yy-mm-dd): 23.09.06

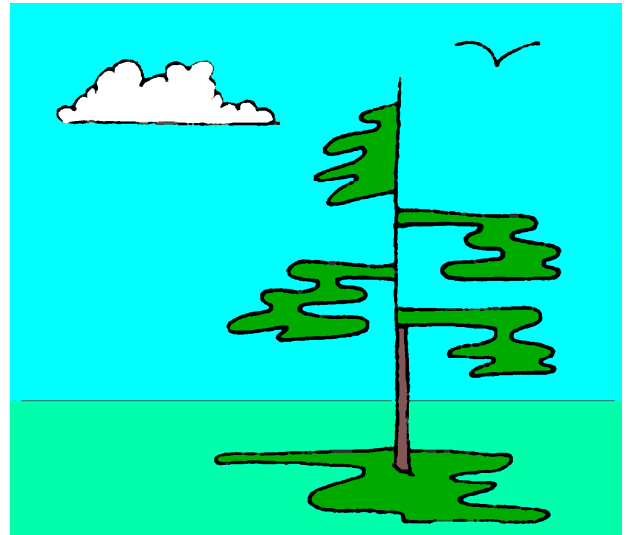
Reference: NBCC, CSA S16

Disclaimer:

The User is responsible to verify data and calculations using an alternative method

Menu:

..... Input Data	 Important Output	 Logical Constructs	Blue Units
..... Sum / For	Red Important Note	 Temporary Variables	



Input Data

Material Property Factors:

$$\phi_s := 0.90$$

$$\phi_w := 0.67$$

Load Factors:

$$\alpha_L := 1.50$$

$$\alpha_D := 1.25$$

Check $(\alpha_D \geq 1.25) = "...OK"$

Check $(\alpha_L \geq 1.5) = "...OK"$

Steel Properties:

W Section: $st_{NDX} := 1$

NDX	des	fy	Fu	
1	"G40.21-350W"	50 Ksi	65 Ksi	$desM_{st1} := st_{NDX} 2$
2	"G40.21-300W"	44 Ksi	65 Ksi	$f_{y1} := st_{NDX} 3$
3	"A36"	36 Ksi	58 Ksi	$F_{u1} := st_{NDX} 4$

$$G_s := \frac{E_s}{2 \cdot (1 + \nu)} \quad \gamma_s := 489 \text{ pcf} \quad \nu := 0.3 \quad E_s := 29000 \text{ Ksi}$$

C Section: $st_{NDX} := 1$

$$desM_{st2} := st_{NDX} 2 \quad f_{y2} := st_{NDX} 3 \quad F_{u2} := st_{NDX} 4$$

Beam Span:

$$L := 12.25 \text{ ft}$$

$$L = 12.25 \text{ ft}$$

$$L = 3.73 \text{ m}$$

Span Length

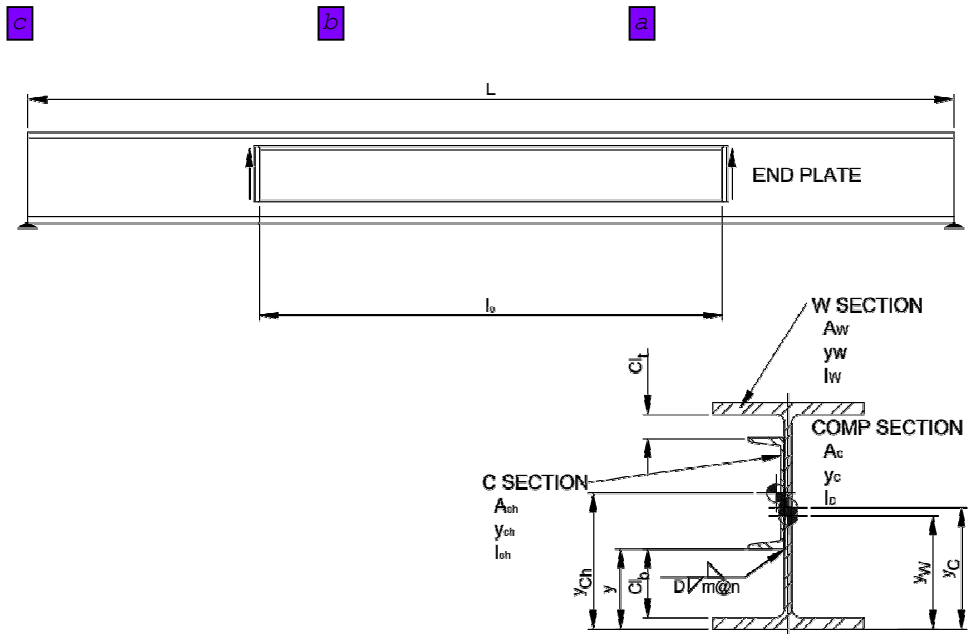
Factored Design Moment:

$$M_f := 250 \text{ K_ft}$$

$$M_f = 250.00 \text{ K_ft}$$

$$M_f = 338.95 \text{ kN_m}$$

Factored Design Moment



Welding:

Weld Electrodes: $we_{NDX} := 2$

NDX	Desl	DesM	UTS	
1	"E60xx"	"E43xx"	60 ksi	$desI_w := we_{NDX}^2$
2	"E70xx"	"E49xx"	70 ksi	$desM_w := we_{NDX}^3$
3	"E80xx"	"E55xx-x"	80 ksi	$X_u := we_{NDX}^4$
4	"E90xx"	"E62xx-x"	90 ksi	

Weld Sizes: $ws_{NDX} := 12$

NDX	desl	desM	D	
1	"	"3mm"	0.11811 in	$desI_D := ws_{NDX}^2$
2	"1/8"	"	0.125 in	$desM_D := ws_{NDX}^3$
3	"	"4mm"	0.15748 in	$D := ws_{NDX}^4$
4	"3/16"	"	0.1875 in	$desI_w = "E70xx"$
5	"	"5mm"	0.19685 in	$desM_w = "E49xx"$
6	"	"6mm"	0.23622 in	$X_u = 70 \text{ ksi}$
7	"1/4"	"	0.25 in	$X_u = 483 \text{ MPa}$
8	"	"7mm"	0.275591 in	
9	"5/16"	"	0.3125 in	
10	"	"8mm"	0.314961 in	
11	"	"9mm"	0.354331 in	
12	"3/8"	"	0.375 in	
13	"	"10mm"	0.3937 in	
14	"	"11mm"	0.4331 in	
15	"7/16"	"	0.4375 in	
16	"	"12mm"	0.4724 in	
17	"1/2"	"	0.50 in	

$m := 12 \text{ in}$

$m = 12.00 \text{ in}$

$m = 304.8 \text{ mm}$

Weld Length

$n := 12 \text{ in}$

$n = 12.00 \text{ in}$

$n = 304.8 \text{ mm}$

Weld Spacing

$y := 1.4 \text{ in}$

$y = 1.40 \text{ in}$

$y = 35.6 \text{ mm}$

Channel Location from Bottom

$Cl_{min} := 3 \text{ in}$

$Cl_{min} = 3.00 \text{ in}$

$Cl_{min} = 76.2 \text{ mm}$

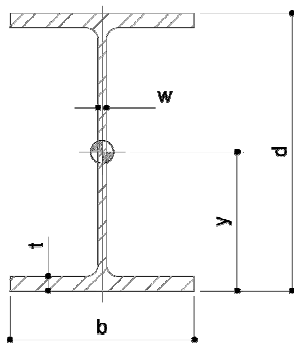
Minimum Clearance for Weld

Section Properties:

W Section: $be_{NDX} := 2$

Section Properties (from CISC Handbook):

NDX	desl	desM	d	b	t	w	k	k1	
1	"W18x46"	"	18.1 in	6.06 in	0.605 in	0.36 in	1.01 in	0.8125 in	$desI_w := be_{NDX}^2$
2	"W16x26"	"	15.7 in	5.50 in	0.345 in	0.25 in	0.747 in	0.75 in	$desM_w := be_{NDX}^3$
3	"W14x26"	"	13.9 in	5.03 in	0.42 in	0.255 in	0.82 in	0.75 in	$d_w := be_{NDX}^4$
4	"W12x14"	"	11.9 in	3.97 in	0.225 in	0.200 in	0.525 in	0.5625 in	
			$b_w := be_{NDX}^5$	$t_w := be_{NDX}^6$	$w_w := be_{NDX}^7$	$k_w := be_{NDX}^8$	$k_{1w} := be_{NDX}^9$		



$desI_w = "W16x26"$

$desM_w = ""$

Designation

$d_w = 15.700 \text{ in}$

$d_w = 398.8 \text{ mm}$

Depth

$b_w = 5.5 \text{ in}$

$b_w = 139.7 \text{ mm}$

Flange Width

$t_w = 0.345 \text{ in}$

$t_w = 8.8 \text{ mm}$

Flange Thickness

$w_w = 0.25 \text{ in}$

$w_w = 6.4 \text{ mm}$

Web Thickness

$k_w = 0.75 \text{ in}$

$k_w = 19 \text{ mm}$

k Distance

$k_{1w} = 0.75 \text{ in}$

$k_{1w} = 19 \text{ mm}$

k1 Distance

$$Y_W := \frac{d_W}{2}$$

$$Y_W = 7.85 \text{ in}$$

$$Y_W = 199.4 \text{ mm}$$

Depth to Outer Fibre

$$A_W := 2 \cdot b_W \cdot t_W + (d_W - 2 \cdot t_W) \cdot w_W$$

$$A_W = 7.55 \text{ in}^2$$

$$A_W = 4869 \text{ mm}^2$$

Area

$$I_{X_W} := \frac{1}{12} \cdot \left(b_W \cdot d_W^3 - (b_W - w_W) \cdot (d_W - 2 \cdot t_W)^3 \right)$$

$$I_{X_W} = 294.18 \text{ in}^4$$

$$I_{X_W} = 1.22 \cdot 10^8 \text{ mm}^4$$

Moment of Inertia (X-Axis)

$$S_{X_W} := \frac{I_{X_W}}{Y_W}$$

$$S_{X_W} = 37.48 \text{ in}^3$$

$$S_{X_W} = 6.14 \cdot 10^5 \text{ mm}^3$$

Section Modulus (X-Axis)

$$Z_{X_W} := \frac{1}{4} \cdot \left(b_W \cdot d_W^2 - (b_W - w_W) \cdot (d_W - 2 \cdot t_W)^2 \right)$$

$$Z_{X_W} = 43.22 \text{ in}^3$$

$$Z_{X_W} = 7.08 \cdot 10^5 \text{ mm}^3$$

Plastic Section (X-Axis)

Property Override:

```
A'_W := 7.68 in^2
if A'_W = 0 in^2
    A_W := A_W
else
    A_W := A'_W
```

c

b

a

$$A_W = 7.68 \text{ in}^2$$

$$A_W = 4955 \text{ mm}^2$$

Design Area

```
Sx'_W := 38.4 in^3
```

```
if Sx'_W = 0 in^3
    Sx_W := Sx_W
else
    Sx_W := Sx'_W
```

$$S_{X_W} = 38.40 \text{ in}^3$$

$$S_{X_W} = 6 \cdot 10^5 \text{ mm}^3$$

Design Section Modulus

```
Ix'_W := 301 in^4
```

```
if Ix'_W = 0 in^4
    Ix_W := Ix_W
else
    Ix_W := Ix'_W
```

$$I_{X_W} = 301.00 \text{ in}^4$$

$$I_{X_W} = 1 \cdot 10^8 \text{ mm}^4$$

Design Moment of Inertia

Class of Section

$$bt_1 := \frac{145}{\sqrt{\frac{f_{y1}}{\text{MPa}}}}$$

$$bt_1 = 7.8095$$

b/t Ratio Class 1

$$bt_2 := \frac{170}{\sqrt{\frac{f_{y1}}{\text{MPa}}}}$$

$$bt_2 = 9.156$$

b/t Ratio Class 2

$$bt_3 := \frac{200}{\sqrt{\frac{f_{y1}}{\text{MPa}}}}$$

$$bt_3 = 10.7717$$

b/t Ratio Class 3

$$dw_1 := \frac{1600}{\sqrt{\frac{f_{y1}}{\text{MPa}}}}$$

$$dw_1 = 86.1738$$

d/w Ratio Class 1

$$dw_2 := \frac{1700}{\sqrt{\frac{f_{y1}}{\text{MPa}}}} \quad dw_2 = 91.5597$$

d/w Ratio Class 2

$$dw_3 := \frac{1900}{\sqrt{\frac{f_{y1}}{\text{MPa}}}} \quad dw_3 = 102.3314$$

d/w Ratio Class 3

```

if  $\left( \left( \frac{b_w}{2 \cdot t_w} \right) \leq bt_1 \right) \wedge \left( \frac{d_w}{w_w} \leq dw_1 \right)$ 
  Classw := 1
else
  if  $\left( \left( \left( \frac{b_w}{2 \cdot t_w} \right) \leq bt_2 \right) \wedge \left( \frac{d_w}{w_w} \leq dw_2 \right) \right)$ 
    Classw := 2
  else
    if  $\left( \left( \frac{b_w}{2 \cdot t_w} \right) \leq bt_3 \right) \wedge \left( \frac{d_w}{w_w} \leq dw_3 \right)$ 
      Classw := 3
    else
      Classw := 4

```

$$Class_w = 2$$

Moment Resistance:

```

if  $(Class_w = 1) \vee (Class_w = 2)$ 
  MrW := φs · fy1 · ZxW
else
  MrW := φs · fy1 · SxW

```

$$M_{rW} = 162.1 \text{ K_ft}$$

$$M_{rW} = 219.7 \text{ kN_m}$$

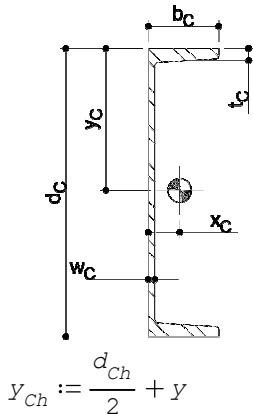
Moment Resistance

C Section: $ch_{NDX} := 1$

Section Properties (from CISC Handbook):

NDX	desI	desM	d	b	t	w
1	"C12x25"	"	12 in	3.05 in	0.501 in	0.387 in
2	"C8x13.7"	"	8 in	2.34 in	0.39 in	0.303 in
3	"C6x10.5"	"	6 in	2.03 in	0.343 in	0.314 in

$$\begin{aligned}
desI_{Ch} &:= ch_{NDX} \ 2 & desM_{Ch} &:= ch_{NDX} \ 3 \\
d_{Ch} &:= ch_{NDX} \ 4 & b_{Ch} &:= ch_{NDX} \ 5 \\
t_{Ch} &:= ch_{NDX} \ 6 & w_{Ch} &:= ch_{NDX} \ 7
\end{aligned}$$



$$desI_{Ch} = \text{"C12x25"}$$

$$desM_{Ch} = \text{" "}$$

$$b_{Ch} = 3.05 \text{ in}$$

$$b_{Ch} = 77.5 \text{ mm}$$

$$d_{Ch} = 12.00 \text{ in}$$

$$d_{Ch} = 304.8 \text{ mm}$$

$$t_{Ch} = 0.50 \text{ in}$$

$$t_{Ch} = 12.7 \text{ mm}$$

$$w_{Ch} = 0.39 \text{ in}$$

$$w_{Ch} = 9.8 \text{ mm}$$

$$y_{Ch} = 7.40 \text{ in}$$

$$y_{Ch} = 188 \text{ mm}$$

Imperial Designation

Metric Designation

Width

Depth

Flange Thickness

Web Thickness

Centroid of Channel from Bot

Weld Clearance:

$$Cl_t := d_w - y - d_{Ch} - t_w$$

$$Cl_t = 1.96 \text{ in}$$

$$Cl_t = 49.7 \text{ mm}$$

Weld Clearance Top

$$Cl_b := y - t_w$$

$$Cl_b = 1.06 \text{ in}$$

$$Cl_b = 26.8 \text{ mm}$$

Weld Clearance Bottom

Channel Properties

$$A_{Ch} := d_{Ch} \cdot w_{Ch} + 2 \cdot (b_{Ch} - w_{Ch}) \cdot t_{Ch}$$

$$A_{Ch} = 7.31 \text{ in}^2$$

$$A_{Ch} = 4718 \text{ mm}^2$$

Area

$$I_{X_{Ch}} := \frac{1}{12} \cdot \left(b_{Ch} \cdot d_{Ch}^3 - (b_{Ch} - w_{Ch}) \cdot (d_{Ch} - 2 \cdot t_{Ch})^3 \right)$$

$$I_{X_{Ch}} = 143.99 \text{ in}^4$$

$$I_{X_{Ch}} = 5.99 \cdot 10^7 \text{ mm}^4$$

Moment of Inertia (X-Axis)

Property Override:

$$A'_{Ch} := 7.32 \text{ in}^2$$

$$\text{if } A'_{Ch} = 0 \text{ in}$$

$$A_{Ch} := A_{Ch}$$

else

$$A_{Ch} := A'_{Ch}$$

$$A_{Ch} = 7.32 \text{ in}^2$$

$$A_{Ch} = 4723 \text{ mm}^2$$

Design Area

$$I_{X'_{Ch}} := 143.9 \text{ in}^4$$

$$\text{if } I_{X'_{Ch}} = 0 \text{ in}^4$$

$$I_{X_{Ch}} := I_{X_{Ch}}$$

else

$$I_{X_{Ch}} := I_{X'_{Ch}}$$

$$I_{X_{Ch}} = 143.90 \text{ in}^4$$

$$I_{X_{Ch}} = 6 \cdot 10^7 \text{ mm}^4$$

Design Moment of Inertia

Composite Section Centroid

$$A_C := A_W + A_{Ch}$$

$$A_C = 15.000 \text{ in}^2$$

$$A_C = 9677 \text{ mm}^2$$

Composite Area

$$Y_{Cb} := \frac{A_W \cdot Y_W + A_{Ch} \cdot Y_{Ch}}{A_C}$$

$$Y_{Cb} = 7.630 \text{ in}$$

$$Y_{Cb} = 193.8 \text{ mm}$$

Composite Centroid Bottom

$$Y_{Ct} := d_W - Y_{Cb}$$

$$Y_{Ct} = 8.070 \text{ in}$$

$$Y_{Ct} = 205.0 \text{ mm}$$

Composite Centroid Top

$$I_{X_C} := I_{X_W} + A_W \cdot (Y_W - Y_{Cb})^2 + I_{X_{Ch}} + A_{Ch} \cdot (Y_{Ch} - Y_{Cb})^2$$

$$I_{X_C} = 446 \text{ in}^4$$

$$I_{X_C} = 1.85 \cdot 10^8 \text{ mm}^4$$

Composite Moment of Inertia

$$S_{X_{Ct}} := \frac{I_{X_C}}{Y_{Ct}}$$

$$S_{X_{Ct}} = 55.2 \text{ in}^3$$

$$S_{X_{Ct}} = 9.05 \cdot 10^5 \text{ mm}^3$$

Composite Sx (Top Fibre)

$$S_{X_{Cb}} := \frac{I_{X_C}}{Y_{Cb}}$$

$$S_{X_{Cb}} = 58.4 \text{ in}^3$$

$$S_{X_{Cb}} = 9.57 \cdot 10^5 \text{ mm}^3$$

Composite Sx (Bottom Fibre)

Moment Resistance as a Class 3 Section:

$$M_{rC3} := \left(\varphi_s \cdot \min \left(\left[\begin{array}{l} f_{y1} \cdot S_{X_{Ct}} \\ f_{y1} \cdot S_{X_{Cb}} \end{array} \right] \right) \right)$$

$$M_{rC3} = 207.10 \text{ K_ft}$$

$$M_{rC3} = 280.79 \text{ kN_m}$$

Moment Resistance (Class 3)

Check Channel Leg and W Section as a Class 2 Section:

$$\text{Check} \left(\left(\text{Class}_W \leq 2 \right) \wedge \left(\frac{b_{Ch}}{t_{Ch}} \leq \frac{170}{\sqrt{\frac{f_{y2}}{\text{MPa}}}} \right) \right) = "...OK"$$

Determine Resisting Class 2 Moment.

$$F_{Wtf} := \varphi_s \cdot f_{y1} \cdot b_W \cdot t_W \quad F_{Wtf} = 85.39 \text{ K}$$

$$F_{Wtf} = 379.82 \text{ kN}$$

$$M_{Wtf} := F_{Wtf} \cdot \left(Y_{Ct} - \frac{t_W}{2} \right) \quad M_{Wtf} = 56.19 \text{ K_ft}$$

$$M_{Wtf} = 76.19 \text{ kN_m}$$

$$F_{Wtw} := \varphi_s \cdot f_{y1} \cdot \left(Y_{Ct} - t_W \right) \cdot w_W \quad F_{Wtw} = 86.90 \text{ K}$$

$$F_{Wtw} = 386.56 \text{ kN}$$

$$M_{Wtw} := F_{Wtw} \cdot \left(\frac{Y_{Ct} - t_W}{2} \right) \quad M_{Wtw} = 27.97 \text{ K_ft}$$

$$M_{Wtw} = 37.92 \text{ kN_m}$$

$$F_{Chtf} := \varphi_s \cdot f_{y2} \cdot b_{Ch} \cdot t_{Ch} \quad F_{Chtf} = 68.76 \text{ K}$$

$$F_{Chtf} = 305.87 \text{ kN}$$

$$M_{Chtf} := F_{Chtf} \cdot \left(Y_{Ct} - t_W - Cl_t - \frac{t_{Ch}}{2} \right) \quad M_{Chtf} = 31.63 \text{ K_ft}$$

$$M_{Chtf} = 42.88 \text{ kN_m}$$

$$F_{Chtw} := \varphi_s \cdot f_{y2} \cdot \left(Y_{Ct} - t_W - Cl_t - t_{Ch} \right) \cdot w_{Ch} \quad F_{Chtw} = 91.75 \text{ K}$$

$$F_{Chtw} = 408.14 \text{ kN}$$

$$M_{Chtw} := F_{Chtw} \cdot \left(\frac{Y_{Ct} - t_W - Cl_t - t_{Ch}}{2} \right) \quad M_{Chtw} = 20.14 \text{ K_ft}$$

$$M_{Chtw} = 27.31 \text{ kN_m}$$

$$A_{sum1} := b_W \cdot t_W + \left(Y_{Ct} - t_W \right) \cdot w_W + b_{Ch} \cdot t_{Ch} + \left(Y_{Ct} - t_W - Cl_t - t_{Ch} \right) \cdot w_{Ch}$$

$$A_{sum1} = 7.40 \text{ in}^2$$

$$A_{sum1} = 4771.38 \text{ mm}^2$$

$$F_{sum1} := F_{Wtf} + F_{Wtw} + F_{Chtf} + F_{Chtw}$$

$$F_{sum1} = 332.80 \text{ K}$$

$$F_{sum1} = 1480.39 \text{ kN}$$

$$F_{Chbw} := \varphi_s \cdot f_{y2} \cdot \left(Y_{Cb} - t_W - Cl_b - t_{Ch} \right) \cdot w_{Ch} \quad F_{Chbw} = 99.78 \text{ K}$$

$$F_{Chbw} = 443.83 \text{ kN}$$

$$M_{Chbw} := F_{Chbw} \cdot \left(\frac{Y_{Cb} - t_W - Cl_b - t_{Ch}}{2} \right) \quad M_{Chbw} = 23.82 \text{ K_ft}$$

$$M_{Chbw} = 32.29 \text{ kN_m}$$

$$F_{Chbf} := \varphi_s \cdot f_{y2} \cdot \left(b_{Ch} \cdot t_{Ch} \right) \quad F_{Chbf} = 68.76 \text{ K}$$

$$F_{Chbf} = 305.87 \text{ kN}$$

$$M_{Chbf} := F_{Chbf} \cdot \left(Y_{Cb} - t_W - Cl_b - \frac{t_{Ch}}{2} \right) \quad M_{Chbf} = 34.27 \text{ K_ft}$$

$$M_{Chbf} = 46.46 \text{ kN_m}$$

$$F_{Wbw} := \varphi_s \cdot f_{y1} \cdot \left(Y_{Cb} - t_W \right) \cdot w_W \quad F_{Wbw} = 81.96 \text{ K}$$

$$F_{Wbw} = 364.58 \text{ kN}$$

$$M_{Wbw} := F_{Wbw} \cdot \left(\frac{Y_{Cb} - t_W}{2} \right) \quad M_{Wbw} = 24.88 \text{ K_ft}$$

$$M_{Wbw} = 33.73 \text{ kN_m}$$

$$F_{Wbf} := \varphi_s \cdot f_{y1} \cdot \left(b_W \cdot t_W \right) \quad F_{Wbf} = 85.39 \text{ K}$$

$$F_{Wbf} = 379.82 \text{ kN}$$

$$M_{Wbf} := F_{Wbf} \cdot \left(Y_{Cb} - \frac{t_W}{2} \right) \quad M_{Wbf} = 53.07 \text{ K_ft}$$

$$M_{Wbf} = 71.95 \text{ kN_m}$$

$$A_{sum2} := (y_{Cb} - t_w - C_{Lb} - t_{Ch}) \cdot w_{Ch} + (b_{Ch} \cdot t_{Ch}) + (y_{Cb} - t_w) \cdot w_w + (b_w \cdot t_w)$$

$$A_{sum2} = 7.46 \text{ in}^2 \quad A_{sum2} = 4815.59 \text{ mm}^2$$

$$F_{sum2} := -F_{Chbw} - F_{Chbf} - F_{Wbw} - F_{Wbf}$$

$$F_{sum2} = -335.89 \text{ K} \quad F_{sum2} = -1494.10 \text{ kN}$$

$$M_{rC2} := M_{Wtf} + M_{Wtw} + M_{Chtf} + M_{Chtw} + M_{Chbw} + M_{Chbf} + M_{Wbw} + M_{Wbf}$$

$$M_{rC2} = 271.96 \text{ K_ft} \quad M_{rC2} = 368.73 \text{ kN_m}$$

Moment Resistance (Class 2)

Bauer's Method:

$$Zx_{C2B} := 71.95 \text{ in}^3$$

$$Zx_{C2B} = 71.95 \text{ in}^3$$

$$Zx_{C2B} = 1179049 \text{ mm}^3$$

Plastic Section Modulus (Bauer)

$$M_{rC2B} := \phi_s \cdot Zx_{C2B} \cdot f_{y1}$$

$$M_{rC2B} = 269.8 \text{ K_ft}$$

$$M_{rC2B} = 368.7 \text{ kN_m}$$

Mom. Resist. (Bauer Class 2)

Weld Capacity:

$$v_{rw} := \phi_w \cdot 0.67 \cdot X_u \cdot \frac{1}{\sqrt{2}}$$

$$v_{rw} = 22.22 \text{ ksi}$$

$$v_{rw} = 153.20 \text{ MPa}$$

Weld Material Strength:

$$v_{rb1} := \phi_w \cdot 0.67 \cdot F_{u1}$$

$$v_{rb1} = 29.18 \text{ Ksi}$$

$$v_{rb1} = 201.18 \text{ MPa}$$

W Section Base Metal Strength

$$v_{rb2} := \phi_w \cdot 0.67 \cdot F_{u2}$$

$$v_{rb2} = 29.18 \text{ Ksi}$$

$$v_{rb2} = 201.18 \text{ MPa}$$

T Section Base Metal Strength

$$V_{rw} := \min \left(\left[v_{rw} \quad v_{rb1} \quad v_{rb2} \right] \right)$$

$$V_{rw} = 22.22 \text{ Ksi}$$

$$V_{rw} = 153.2 \text{ MPa}$$

Minimum Weld Capacity

Maximum UDL:

$$q_f := \frac{8 \cdot M_{rC2}}{L^2}$$

$$q_f = 14.5 \text{ Klf}$$

$$q_f = 211.6 \text{ kNpm}$$

Maximum UDL

$$Q_f := q_f \cdot L$$

$$Q_f = 177.6 \text{ K}$$

$$Q_f = 790.0 \text{ kN}$$

Max Load based on UDL

$$part := 50 \%$$

Part Override:

$$part' := 0 \%$$

$$\text{if } part' = 0 \%$$

$$part := part$$

$$\text{else}$$

$$part := part'$$

$$part = 0.5000$$

Maximum Reaction (Shear):

$$V_f := Q_f \cdot part$$

$$V_f = 88.8 \text{ K}$$

$$V_f = 395.0 \text{ kN}$$

Length of Reinforcing:

$$\Delta M := M_{rC2} - M_{rW}$$

$$\Delta M = 109.90 \text{ K_ft}$$

$$\Delta M = 149.00 \text{ kN_m}$$

Moment Prov by Composite

$$l_C := \sqrt{\frac{8 \cdot \Delta M}{q_f}}$$

$$l_C = 7.7871 \text{ ft}$$

$$l_C = 7.7871 \text{ ft}$$

Length of Reinforcing

Shear Stress at W and Ch Junction

$$A' := A_{Ch}$$

$$A' = 7.32 \text{ in}^2$$

$$A' = 4723 \text{ mm}^2$$

Area

$$y_{bar} := \frac{d_{Ch}}{2} + y$$

$$y_{bar} = 7.40 \text{ in}$$

$$y_{bar} = 188.0 \text{ mm}$$

Centroid of Area

$$\tau := \frac{V_f}{I_{x_C} \cdot d_{Ch}} \cdot A' \cdot (y_{bar})$$

$$\tau = 0.90 \text{ Ksi}$$

$$\tau = 6.2 \text{ MPa}$$

Shear Stress at Ch-W Interface

Connection Shear per Unit Length

$$\tau' := \tau \cdot n \cdot d_{Ch}$$

$$\tau' = 129.5 \text{ K}$$

$$\tau' = 576.2 \text{ kN}$$

Weld Provided

$$\tau_r := 2 \cdot V_{rw} \cdot D \cdot m$$

$$\tau_r = 200.0 \text{ K}$$

$$\tau_r = 889.5 \text{ kN}$$

End Connection Force:**c****b****a**

$$\sigma_1 := \frac{-M_{rc2}}{I_{xc}} \cdot (y + d_{ch} - y_{cb})$$

$$\sigma_1 = -42.3 \text{ Ksi}$$

$$\sigma_1 = -291.3 \text{ MPa}$$

At the Bottom Fibre of Channel

$$\sigma_2 := \frac{-M_{rc2}}{I_{xc}} \cdot (y + d_{ch} - t_{ch} - y_{cb})$$

$$\sigma_2 = -38.6 \text{ Ksi}$$

$$\sigma_2 = -266.0 \text{ MPa}$$

At the Bottom Web of Channel

$$\sigma_3 := \frac{-M_{rc2}}{I_{xc}} \cdot (d_w - y_{ct} - y_{cb})$$

$$\sigma_3 = 2.0 \cdot 10^{-13} \text{ Ksi}$$

$$\sigma_3 = 1.4 \cdot 10^{-12} \text{ MPa}$$

At the Centroid of Channel

$$\sigma_4 := \frac{-M_{rc2}}{I_{xc}} \cdot (y + t_{ch} - y_{cb})$$

$$\sigma_4 = 42.0 \text{ Ksi}$$

$$\sigma_4 = 289.3 \text{ MPa}$$

At the Top Web of Channel

$$\sigma_5 := \frac{-M_{rc2}}{I_{xc}} \cdot (y - y_{cb})$$

$$\sigma_5 = 45.6 \text{ Ksi}$$

$$\sigma_5 = 314.6 \text{ MPa}$$

At the Top Fibre of Channel

$$F_1 := \frac{\sigma_1 + \sigma_2}{2} \cdot b_{ch} \cdot t_{ch}$$

$$F_1 = -61.7582 \text{ K}$$

$$F_1 = -274.7 \text{ kN}$$

$$M_1 := F_1 \cdot \left(d_{ch} - \frac{t_{ch}}{2} \right)$$

$$M_1 = -60.4690 \text{ K_ft}$$

$$M_1 = -81.98 \text{ kN_m}$$

$$F_2 := \frac{\sigma_2 + \sigma_3}{2} \cdot (y_{ct} - t_w - cl_t - t_{ch}) \cdot w_{ch}$$

$$F_2 = -39.3333 \text{ K}$$

$$F_2 = -175.0 \text{ kN}$$

$$M_2 := F_2 \cdot \left((y_{cb} - t_w - cl_b) + 2 \cdot \frac{(y_{ct} - t_w - cl_t - t_{ch})}{3} \right)$$

$$M_2 = -31.9347 \text{ K_ft}$$

$$M_2 = -43.30 \text{ kN_m}$$

$$F_3 := \frac{\sigma_3 + \sigma_4}{2} \cdot (y_{cb} - t_w - cl_b - t_{ch}) \cdot w_{ch}$$

$$F_3 = 46.5145 \text{ K}$$

$$F_3 = 206.9 \text{ kN}$$

$$M_3 := F_3 \cdot \left(t_{ch} + \frac{(y_{cb} - t_w - cl_b - t_{ch})}{3} \right)$$

$$M_3 = 9.3448 \text{ K_ft}$$

$$M_3 = 12.67 \text{ kN_m}$$

$$F_4 := \frac{\sigma_4 + \sigma_5}{2} \cdot b_{ch} \cdot t_{ch}$$

$$F_4 = 66.9145 \text{ K}$$

$$F_4 = 297.7 \text{ kN}$$

$$M_4 := F_4 \cdot \frac{t_{ch}}{2}$$

$$M_4 = 1.3968 \text{ K_ft}$$

$$M_4 = 1.89 \text{ kN_m}$$

$$M_{fch} := M_1 + M_2 + M_3 + M_4$$

$$M_{fch} = -81.6621 \text{ K_ft}$$

$$M_{fch} = -110.72 \text{ kN_m}$$

Unbalanced Moment in Channel

$$F_{fE} := \frac{M_{fch} \cdot 2}{l_c}$$

$$F_{fE} = -20.9737 \text{ K}$$

$$F_{fE} = -93.3 \text{ kN}$$

End Plate Connection Force

Summary:

Material Property Factor

$$(\text{Welds}) \quad \phi_w = 0.67$$

$$(\text{Steel}) \quad \phi_s = 0.9$$

$$\text{Dead Load Factor} \quad \alpha_D = 1.25$$

$$\text{Live Load Factor} \quad \alpha_L = 1.5$$

$$\text{Check } (\alpha_D \geq 1.25) = "...OK"$$

$$\text{Check } (\alpha_L \geq 1.5) = "...OK"$$

Steel Properties

W Section

Designation

Yield Strength

Ultimate Strength

C Section

Designation

Yield Strength

Ultimate Strength

Young's Modulus

Shear Modulus

Density

Beam Span Length

Factored Design Moment (Arbitrary)

Welds

Designation (Welding Electrode)

Ultimate Strength (Welding Electrode)

Designation (Fillet Weld Size)

Fillet Weld Size

Weld Length

Weld Spacing

$$\text{Check } ((m \geq 1.5 \text{ in}) \wedge (m \geq 4 \cdot D)) = "...OK"$$

$$\text{Check } ((n - m) \leq 12 \text{ in}) = "...OK"$$

$$\text{Check } ((n - m) \leq 18 \text{ in}) = "...OK"$$

W Section

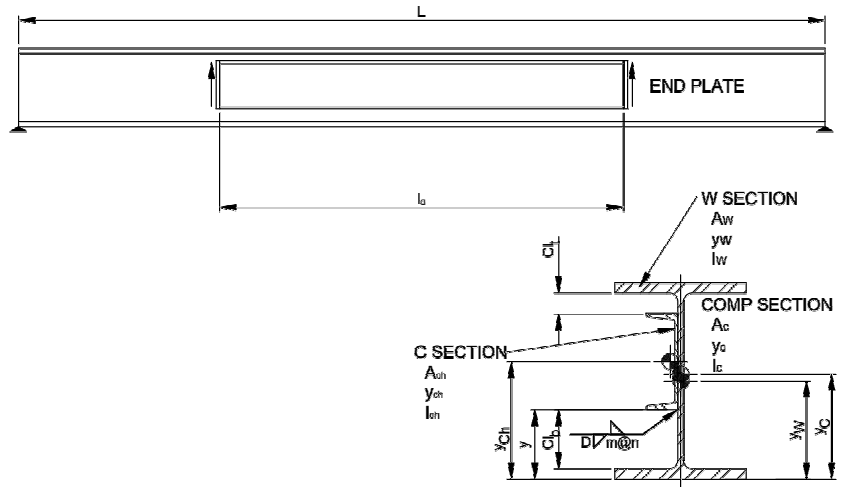
Designation

Depth

Flange Width

Flange Thickness

Web Thickness



$$desM_{st1} = "G40.21-350W"$$

$$f_{y1} = 50 \text{ Ksi}$$

$$f_{y1} = 345 \text{ MPa}$$

$$F_{u1} = 65 \text{ Ksi}$$

$$F_{u1} = 448 \text{ MPa}$$

$$desM_{st2} = "G40.21-350W"$$

$$f_{y2} = 50 \text{ Ksi}$$

$$f_{y2} = 345 \text{ MPa}$$

$$F_{u2} = 65 \text{ Ksi}$$

$$F_{u2} = 448 \text{ MPa}$$

$$E_s = 29000 \text{ Ksi}$$

$$E_s = 1.9995 \cdot 10^5 \text{ MPa}$$

$$G_s = 11153.8462 \text{ Ksi}$$

$$G_s = 76903 \text{ MPa}$$

$$\gamma_s = 489 \text{ pcf}$$

$$\gamma_s = 76.8 \text{ kNpcm}$$

$$L = 12.25 \text{ ft}$$

$$L = 3.73 \text{ m}$$

$$M_f = 250.00 \text{ K_ft}$$

$$M_f = 338.95 \text{ kN_m}$$

$$desI_w = "E70xx"$$

$$desM_w = "E49xx"$$

$$X_u = 70 \text{ Ksi}$$

$$X_u = 483 \text{ MPa}$$

$$desI_D = "3/8"$$

$$desM_D = ""$$

$$D = 0.375 \text{ in}$$

$$D = 9.525 \text{ mm}$$

$$m = 12 \text{ in}$$

$$m = 304.8 \text{ mm}$$

$$n = 12 \text{ in}$$

$$n = 304.8 \text{ mm}$$

$$\leq 12" \text{ for Compression Attachment}$$

$$\leq 18" \text{ for Tension Attachment}$$

$$desI_w = "W16x26"$$

$$desM_w = ""$$

$$d_w = 15.70 \text{ in}$$

$$d_w = 398.8 \text{ mm}$$

$$b_w = 5.5 \text{ in}$$

$$b_w = 139.7 \text{ mm}$$

$$t_w = 0.345 \text{ in}$$

$$t_w = 8.8 \text{ mm}$$

$$w_w = 0.25 \text{ in}$$

$$w_w = 6.4 \text{ mm}$$

k Distance

$$k_W = 0.75 \text{ in}$$

$$k_W = 19 \text{ mm}$$

k1 Distance

$$k_{1W} = 0.75 \text{ in}$$

$$k_{1W} = 19 \text{ mm}$$

Area

$$A_W = 7.68 \text{ in}^2$$

$$A_W = 4954.8 \text{ mm}^2$$

Moment of Inertia (x-axis)

$$I_{x_W} = 301.00 \text{ in}^4$$

$$I_{x_W} = 1.25 \cdot 10^8 \text{ mm}^4$$

Centroid Distance

$$y_W = 7.85 \text{ in}$$

$$y_W = 199.4 \text{ mm}$$

Class of Section

$$Class_W = 2$$

Moment Resistance

$$M_{rW} = 162.1 \text{ K_ft}$$

$$M_{rW} = 219.7 \text{ kN_m}$$

C Section

Designation

$$desI_{Ch} = "C12 \times 25"$$

$$desM_{Ch} = ""$$

Depth

$$d_{Ch} = 12.00 \text{ in}$$

$$d_{Ch} = 304.8 \text{ mm}$$

Web Width

$$w_{Ch} = 0.39 \text{ in}$$

$$w_{Ch} = 9.8 \text{ mm}$$

Area

$$A_{Ch} = 7.32 \text{ in}^2$$

$$A_{Ch} = 4722.6 \text{ mm}^2$$

Moment of Inertia (x-axis)

$$I_{x_{Ch}} = 143.90 \text{ in}^4$$

$$I_{x_{Ch}} = 5.99 \cdot 10^7 \text{ mm}^4$$

Centroid Distance

$$y_{Ch} = 7.40 \text{ in}$$

$$y_{Ch} = 188 \text{ mm}$$

Channel Location from Bottom

$$y = 1.40 \text{ in}$$

$$y = 35.6 \text{ mm}$$

Minimum Clearance for Weld

$$Cl_{min} = 3.00 \text{ in}$$

$$Cl_{min} = 76.2 \text{ mm}$$

Weld Clearance Top

$$Cl_t = 1.96 \text{ in}$$

$$Cl_t = 49.7 \text{ mm}$$

$$Check \left((Cl_t) \geq Cl_{min} \right) = "...NG"$$

Weld Clearance Bottom

$$Cl_b = 1.06 \text{ in}$$

$$Cl_b = 26.8 \text{ mm}$$

$$Check \left(Cl_b \geq Cl_{min} \right) = "...NG"$$

Check Channel Leg and W Section as a Class 2 Section:

$$Check \left((Class_W \leq 2) \wedge \left(\frac{b_{Ch}}{t_{Ch}} \leq \frac{170}{\sqrt{\frac{f_{y2}}{\text{MPa}}}} \right) \right) = "...OK"$$

Composite Section

Area

$$A_C = 15 \text{ in}^2$$

$$A_C = 9677 \text{ mm}^2$$

Moment of Inertia (x-axis)

$$I_{x_C} = 446 \text{ in}^4$$

$$I_{x_C} = 1.85 \cdot 10^8 \text{ mm}^4$$

Centroid Distance (top)

$$y_{Ct} = 8.1 \text{ in}$$

$$y_{Ct} = 205 \text{ mm}$$

Centroid Distance (bot)

$$y_{Cb} = 7.6 \text{ in}$$

$$y_{Cb} = 194 \text{ mm}$$

Section Modulus (design x-axis)

$$S_{x_{Ct}} = 55.2 \text{ in}^3$$

$$S_{x_{Ct}} = 9.05 \cdot 10^5 \text{ mm}^3$$

Section Modulus (design x-axis)

$$S_{x_{Cb}} = 58.4 \text{ in}^3$$

$$S_{x_{Cb}} = 9.57 \cdot 10^5 \text{ mm}^3$$

Moment Resistance (Class 3)

$$M_{rC3} = 207.1 \text{ K_ft}$$

$$M_{rC3} = 280.8 \text{ kN_m}$$

$$Check \left(M_{rC3} \geq M_f \right) = "...NG"$$

$$Check \left(\frac{M_{rC3}}{\phi_s} \geq M_f \right) = "...NG"$$

Allow for Overload

Moment Resistance (Class 2)

$$M_{rC2} = 272.0 \text{ K_ft}$$

$$M_{rC2} = 368.7 \text{ kN_m}$$

$$\text{Check } (M_{rC2} \geq M_f) = "...OK"$$

$$\text{Check } \left(\frac{M_{rC2}}{\phi_s} \geq M_f \right) = "...OK"$$

Allow for Overload

Moment Resistance (Bauer Class 2)

Plastic Section Modulus (Bauer)

$$Z_{xC2B} = 71.95 \text{ in}^3$$

$$Z_{xC2B} = 1179049 \text{ mm}^3$$

Mom. Resist. (Bauer Class 2)

$$M_{rC2B} = 269.8 \text{ K_ft}$$

$$M_{rC2} = 368.7 \text{ kN_m}$$

Maximum UDL

$$q_f = 14.5 \text{ Klf}$$

$$q_f = 211.6 \text{ kNpm}$$

Maximum Reaction (Shear)

$$V_f = 88.8 \text{ K}$$

$$V_f = 395.0 \text{ kN}$$

Length of Reinforcing

$$l_c = 7.7871 \text{ ft}$$

$$l_c = 2.3735 \text{ m}$$

$$\text{Check } (l_c \leq L) = "...OK"$$

Shear Stress at W and C Junction based on Max Shear

$$\tau = 0.90 \text{ Ksi}$$

$$\tau = 6.2 \text{ MPa}$$

Minimum Weld Capacity

$$V_{rw} = 22.22 \text{ Ksi}$$

$$V_{rw} = 153.2 \text{ MPa}$$

Connection Shear per Unit Length

$$\tau' = 129.5 \text{ K}$$

$$\tau' = 576.2 \text{ kN}$$

Minimum Weld Capacity

$$V_{rw} = 22.22 \text{ Ksi}$$

$$V_{rw} = 153.20 \text{ MPa}$$

Weld Resistance per Unit Length

$$\tau_r = 200 \text{ K}$$

$$\tau_r = 889.5 \text{ kN}$$

$$\text{Check } (\tau_r \geq \tau') = "...OK"$$

$$\text{Check } \left(\frac{\tau_r}{\phi_s} \geq \tau' \right) = "...OK"$$

Allow for Overload

End Connection Force:

At the Bottom Fibre of Channel

$$\sigma_1 = -42.3 \text{ Ksi}$$

$$\sigma_1 = -291.3 \text{ MPa}$$

At the Bottom Web of Channel

$$\sigma_2 = -38.6 \text{ Ksi}$$

$$\sigma_2 = -266.0 \text{ MPa}$$

At the Centroid of Channel

$$\sigma_3 = 2.0 \cdot 10^{-13} \text{ Ksi}$$

$$\sigma_3 = 1.4 \cdot 10^{-12} \text{ MPa}$$

At the Top Web of Channel

$$\sigma_4 = 42.0 \text{ Ksi}$$

$$\sigma_4 = 289.3 \text{ MPa}$$

At the Top Fibre of Channel

$$\sigma_5 = 45.6 \text{ Ksi}$$

$$\sigma_5 = 314.6 \text{ MPa}$$

Unbalanced Moment in Channel

$$M_{fCh} = -81.7 \text{ K_ft}$$

$$M_{fCh} = -110.7 \text{ kN_m}$$

Effect of Composite Weld not Included

End Connection Force

$$F_{fE} = -21.0 \text{ K}$$

$$F_{fE} = -93.3 \text{ kN}$$